



Feng, S., Vardanega, P. J., Ibraim, E., Widyatmoko, I., Ojum, C., O'Kelly, B. C., & Nogal, M. (2020). Permeability assessment of some granular mixtures. *Géotechnique*, 70(9), 845–847.  
<https://doi.org/10.1680/jgeot.19.D.005>

Peer reviewed version

Link to published version (if available):  
[10.1680/jgeot.19.D.005](https://doi.org/10.1680/jgeot.19.D.005)

[Link to publication record in Explore Bristol Research](#)  
PDF-document

This is the author accepted manuscript (AAM). The final published version (version of record) is available online via ICE Publishing at <https://www.icevirtuallibrary.com/doi/10.1680/jgeot.19.D.005>. Please refer to any applicable terms of use of the publisher.

## University of Bristol - Explore Bristol Research

### General rights

This document is made available in accordance with publisher policies. Please cite only the published version using the reference above. Full terms of use are available:  
<http://www.bristol.ac.uk/red/research-policy/pure/user-guides/ebr-terms/>

# Accepted manuscript doi: 10.1680/jgeot.19.d.005

---

## **Accepted manuscript**

As a service to our authors and readers, we are putting peer-reviewed accepted manuscripts (AM) online, in the Ahead of Print section of each journal web page, shortly after acceptance.

## **Disclaimer**

The AM is yet to be copyedited and formatted in journal house style but can still be read and referenced by quoting its unique reference number, the digital object identifier (DOI). Once the AM has been typeset, an 'uncorrected proof' PDF will replace the 'accepted manuscript' PDF. These formatted articles may still be corrected by the authors. During the Production process, errors may be discovered which could affect the content, and all legal disclaimers that apply to the journal relate to these versions also.

## **Version of record**

The final edited article will be published in PDF and HTML and will contain all author corrections and is considered the version of record. Authors wishing to reference an article published Ahead of Print should quote its DOI. When an issue becomes available, queuing Ahead of Print articles will move to that issue's Table of Contents. When the article is published in a journal issue, the full reference should be cited in addition to the DOI.

**Manuscript title:** Discussion: Permeability assessment of some granular mixtures

**Authors:** Shuyin Feng\*, Paul J. Vardanega\*, Erdin Ibraim\*, Iswandaru Widyatmoko<sup>†</sup>, Chibuzor Ojum<sup>†</sup>, Brendan C. O’Kelly<sup>‡</sup> and Maria Nogal<sup>‡</sup>

**Affiliations:** \*Department of Civil Engineering, University of Bristol, Bristol, UK;

<sup>†</sup>Pavement Engineering, Centre of Excellence for Asset Consultancy, AECOM, Nottingham, UK and <sup>‡</sup>Department of Civil, Structural and Environmental Engineering, Trinity College Dublin, Dublin 2, Dublin, Ireland.

**Corresponding author:** Paul J. Vardanega, Department of Civil Engineering, University of Bristol, Bristol, BS8 1TR, UK.

**E-mail:** p.j.vardanega@bristol.ac.uk

**Contribution by Brendan C. O’Kelly and Maria Nogal**

As part of their paper (Feng *et al.*, 2019), the authors presented the novel application of the grading entropy framework for hydraulic conductivity assessments, along with a model (Equation (27)), requiring only the normalised grading entropy coordinates  $A$  and  $B$  as inputs, for reliably predicting the permeability coefficient ( $k_{20^\circ\text{C}}$ ) values of 30 compacted crushed basalt–gritstone gravel mixtures investigated. The discussers note that the gradation characteristics of these gravel mixtures were such that their  $B$  values negatively correlate with  $A$  ( $R^2 = 0.50$ ), meaning that the  $k_{20^\circ\text{C}}$  values could also be reasonably predicted based entirely on their  $A$  values, or less reliably based solely on their  $B$  values (Equations (25) and (26), respectively). Further, considering all 30 gravel mixtures investigated, their reported values of void ratio ( $e$ ) linearly correlate positively and negatively with  $A$  ( $R^2 = 0.64$ ) and  $B$  ( $R^2 = 0.53$ ), respectively. Importantly, the values of coefficients deduced for these various correlations are dependent on compaction level. As such, Equations (25)–(27) would generally underestimate the actual  $k_{20^\circ\text{C}}$  values for the same gravel mixtures placed at lower densification levels (higher  $e$  values). In these instances, the inclusion of the  $e$  parameter in the model (Equation (27)) would seem appropriate, thereby extending its scope and reliability for other field applications (e.g. assessing the loosely placed materials as potential drainage/filter media). Therefore, the following regression model is proposed:

$$k_{20^\circ\text{C}} = C_1 A^{C_2} B^{C_3} e^{C_4} \quad (28)$$

where  $C_1$ ,  $C_2$ ,  $C_3$  and  $C_4$  are the fitting coefficients. Note that  $C_1$  is expressed in the same units as  $k_{20^\circ\text{C}}$ , whereas the other coefficients are dimensionless. This avoids the mathematical and physical inconsistencies discussed in Castillo *et al.* (2014a, 2014b).

Towards demonstrating this point, the discussers performed multiple linear regression analysis for the proposed model (Equation (28)) utilising the listed  $A$ ,  $B$  and  $e$  values for the various granular mixtures presented in Table 2. Compared to the authors’ model (Equation (27):  $R^2 = 0.90$ ,  $n = 30$ ,  $p < 0.0001$ ), the analysis for the proposed expanded model with deduced fitting coefficients  $C_1 = 662.75$  mm/s,  $C_2 = 5.55$ ,  $C_3 = -1.32$  and  $C_4 = 4.58$  resulted in a slightly better fit ( $R^2 = 0.96$ ,  $n = 30$ ,  $p < 0.0001$ ). In terms of the adjusted R-squared,  $R_a^2$ , which penalises the number of predictors employed in the model, the two-variable model exhibits a value of 0.88, compared to the proposed three-variable model  $R_a^2$  value of 0.95, again indicative of the latter model’s better fit.

In order to further test the advantage of the proposed expanded model for other soil types (classifications), the discussers employed the same investigative approach in considering the dataset comprising of  $A$ ,  $B$ ,  $e$  and  $k_{20^\circ\text{C}}$  values reported for 20 silty sand and sand materials in the paper by Arshad *et al.* (2019). Compared to the gravel mixtures investigated by the authors ( $D_{10} = 0.72$ – $7.02$  mm,  $e = 0.51$ – $0.85$  and  $k_{20^\circ\text{C}} = 4.19$ – $561.20$  mm/s), these sandy soils had particle shape classes varying from angular to sub-angular,  $D_{10} = 0.01$ – $0.50$  mm,  $e = 0.32$ – $0.60$  and substantially lower values of  $k_{20^\circ\text{C}}$  ranging 0.0007–3.50 mm/s. Further, in terms of linear correlation, the  $B$

values of these sandy soils only weakly correlated with their  $A$  values ( $R^2 = 0.27$ ), such that when analysed independently,  $k_{20^\circ\text{C}}$  only weakly correlated with  $A$  and  $B$ . Overall, the same general trends are evident for both Arshad *et al.* (2019) and Feng *et al.* (2019) datasets, namely:  $\log k_{20^\circ\text{C}}$  correlates positively with  $\log A$  and negatively with  $\log B$ , while  $\log D_{10}$  and  $e$  both correlate positively with  $A$  and negatively with  $B$ . In terms of the  $k_{20^\circ\text{C}} - e$  relationship: for the Arshad *et al.* (2019) dataset, power fitting produced superior  $R^2$ , whereas for the Feng *et al.* (2019) dataset, power and exponential fitting were found to produce comparable results ( $R^2 \approx 0.86$ ).

The discussers found that Equation (27) cannot reliably represent the described sandy soil dataset, with 90% of the Arshad *et al.* (2019) points falling outside the prediction intervals ( $\alpha = 5\%$ ). Further, with fitting coefficient values deduced for the basalt–gritstone gravel mixtures as inputs, the proposed expanded model also cannot reliably represent the sandy soil dataset, with 95% of the Arshad *et al.* (2019) points falling outside the prediction intervals ( $\alpha = 5\%$ ). In other words, it would appear that the deduced fitting coefficient values are specific to the particular test material under investigation. This is not unexpected, but consistent with the fact that the two samples (datasets) do not represent the same statistical population.

The discussers also performed multiple linear regression analysis of the dataset ( $n = 20$ ) for the silty sand and sand materials to investigate the goodness of fit achieved for both prediction models. Compared to the model without the  $e$  parameter included ( $R^2 = 0.45$ ,  $R_a^2 = 0.30$ , and  $p = 0.006$  for  $C_4 = 0$ ), the proposed model (Equation (28)) produced values of  $R^2 = 0.68$ ,  $R_a^2 = 0.56$  and  $p = 0.0003$ . That is, consideration of the  $e$  parameter significantly improves the fitting, with less than 50% of the variability of the data predicted by the two-variable model, whereas Equation (28) accounts for almost 70% of the variability of the model. Further, the  $k_{20^\circ\text{C}}$  values for these sandy soils were mostly captured by  $B$  and  $e$ , whereas  $A$  adjusts the final  $k_{20^\circ\text{C}}$  value (small  $C_2$  value). Compared to the basalt–gritstone gravel mixtures, the substantially different values of the deduced fitting coefficients and significantly lower  $R^2$  values for the sandy soils may be explained by greater variation in their gradation characteristics (with  $B$  substantially independent of  $A$ ), differences in shape factor for the solid particles and their significantly greater specific surface area ( $S_s$ ), as well as possibly the relatively small sample size of this dataset. As with the evolution of conventional permeability models, the inclusion of a particle shape factor and the  $S_s$  parameter in the formulation of the discussed models may further enhance their performance.

---

**Authors' reply**

The authors are delighted by the discussers' interest in the research presented in Feng et al. (2019). The discussers have proposed an improvement to a regression model, Eq. 27 (Feng et al., 2019), for the hydraulic conductivity (or coefficient of permeability) which considers the compaction level of soil mixtures (Eq. 28). As an indicator for compaction level, the void ratio ( $e$ ) is now incorporated along with the normalised grading entropy coordinates  $A$  and  $B$  as predictors of permeability. Eq. 27 and Eq. 28 were then examined by the discussers using the dataset of gravel mixtures from Feng et al. (2019) ( $n = 30$ ) and the silty-sand and sand materials dataset ( $n = 20$ ) from Arshad et al. (2019). The authors agree that the inclusion of the void ratio ( $e$ ) does enhance the prediction accuracy of Eq. 27 to some extent as illustrated by the discussers in their contribution. This is expected as the void ratio ( $e$ ) has been long recognised as a primary predictor for coefficient of permeability of soils and soil mixtures and is included in many empirical and semi-empirical models (e.g., Kozeny 1927; Carmen 1937, 1939; Carrier, 2003; Chapuis and Aubertin, 2003; Chapuis, 2004; Chapuis 2012 and Vardanega et al. 2017).

The authors have further refined the fitting coefficients  $C_1$  to  $C_4$  in Eq. 28 given by the discussers using the original data files from Feng et al. (2019), to generate Eq. (28a):

$$k_{20^\circ\text{C}}(\text{mm/s}) = 671.83A^{5.59}B^{-1.30}e^{4.58} \quad R^2 = 0.96, n = 30, p < 0.0001 \quad (28a)$$

The slight discrepancy between Eq. 28 and Eq. 28a (i.e. the values of  $C_1$  to  $C_4$ ) is because the data presented in the technical note was rounded to two decimal places to save space. To further illustrate the improvement in prediction of the coefficient of permeability for the dataset from Feng et al. (2019) ( $n = 30$ ) using Eq. 28a instead of Eq. 27, the predicted against measured value plot (Fig. 13) was drawn. Compared to the predicted versus measured plot generated using Eq. 27 (Fig. 12, Feng et al., 2019), it can be seen that a few more points migrate into the  $\pm 50\%$  bounds. The authors therefore agree with the discussers that Eq. 28a has better predictive power than Eq. 27 for this dataset. It is also worth pointing out that Fig. 13 has the more points within the  $\pm 50\%$  bounds than shown on Fig. 8 (i.e. for the 'Kozeny-Carman' model (Kozeny 1927; Carmen 1937, 1939). Therefore, for the dataset from Feng et al. (2019) ( $n = 30$ ) Eq. 28a is the most preferred choice (of the models examined) on the basis of both the coefficient of determination values and the number of points falling within the  $\pm 50\%$  bounds on the predicted versus measured plots.

That said the authors wish to re-iterate that the aim of the research presented in Feng et al. (2019) was to determine if the normalised grading entropy coordinates alone can be used to give an estimate of the coefficient of permeability of gravel mixtures. Pavement engineers often specify gradation curve envelopes for mixtures when designing such mixtures and therefore a method to estimate coefficient of permeability straight from the gradation curve itself is useful. To this end, Eq. 27 did not include void ratio: the effect of void ratio having been tested via the ‘Kozeny-Carman’ (e.g., Kozeny 1927; Carmen 1937, 1939) and Chapuis (2012) formulations (see Figs 7 and 8) in the original technical note. However, the authors agree that forms of Eq. 28 should be considered for future research work in this area.

The discussers also note that Eq. 27 cannot successfully predict the data-set presented in Arshad et al. (2019) ( $n = 20$ ). As Eq. 27 is calibrated based on the data of one soil type only this is not surprising. The authors never suggested that Eq. 27 was a universal equation for all soils or soil mixtures. As stated in Feng et al. (2019), “... the coefficients and correlations calculated in equations (18)-(27) only hold for the specific soil tested in this research...” With a larger database of more diverse soil types and gradations, the calibrated coefficients of Eq. 27 may be made more representative. It is good to read that the discussers report similar overall trends for the Arshad et al. (2019) dataset as for the dataset from Feng et al. (2019). This further confirms that some degree of correlation exists between the coefficient of permeability and the normalised grading entropy co-ordinates.

The authors agree with the discussers that inclusion of more information such as specific surface and particle shape factor should improve the correlations. However, as already stated, the authors were attempting to see how well a smaller number of parameters could predict the coefficient of permeability of the gravel mixtures studied.

The authors would like to restate their appreciation to the discussers for their effort in producing Eq. (28) and re-examining the two data-sets presented in Feng et al. (2019) and Arshad et al. (2019), although predicted versus measured plots similar to those shown in Fig. 13 for the dataset in Arshad et al. (2019) would be helpful to further demonstrate graphically the predictive power of the fittings presented by the discussers.

## References

- Arshad, M., Nazir, M. S. and O'Kelly, B. C. (2019). Evolution of hydraulic conductivity models for sandy soils. *Proceedings of the Institution of Civil Engineers – Geotechnical Engineering*, <https://doi.org/10.1680/jgeen.18.00062>
- Carman, P. C. (1937). Fluid flow through granular beds. *Transactions-Institution of Chemical Engineers*, **15**: 150-166.
- Carman, P. C. (1939). Permeability of saturated sands, soils and clays. *The Journal of Agricultural Science*, **29(2)**: 262–273, <https://doi.org/10.1017/S0021859600051789>
- Carrier, W. D. III (2003). Goodbye, Hazen; Hello, Kozeny-Carman. *Journal of Geotechnical and Geoenvironmental Engineering*, **129(11)**: 1054-1056, [http://dx.doi.org/10.1061/\(ASCE\)1090-0241\(2003\)129:11\(1054\)](http://dx.doi.org/10.1061/(ASCE)1090-0241(2003)129:11(1054))
- Castillo, E., Calviño, A., Nogal, M., & Lo, H. K. (2014a). On the probabilistic and physical consistency of traffic random variables and models. *Computer-Aided Civil and Infrastructure Engineering* **29**, No. 7, 496–517, <https://doi.org/10.1111/mice.12061>.
- Castillo, E., O'Connor, A. J., Nogal, M., & Calviño, A. (2014b). On the physical and probabilistic consistency of some engineering random models. *Structural Safety*, **51**, 1–12, <http://dx.doi.org/10.1016/j.strusafe.2014.05.003>.
- Chapuis, R. P. (2004). Predicting the saturated hydraulic conductivity of sand and gravel using effective diameter and void ratio. *Canadian Geotechnical Journal*, **41(5)**: 787-795, <https://doi.org/10.1139/t04-022>
- Chapuis, R. P. (2012). Predicting the saturated hydraulic conductivity of soils: a review. *Bulletin of Engineering Geology and the Environment*, **71(3)**: 401–434, <https://doi.org/10.1007/s10064-012-0418-7>
- Chapuis, R. P., and Aubertin, M. (2003). On the use of the Kozeny Carman equation to predict the hydraulic conductivity of soils. *Canadian Geotechnical Journal*, **40(3)**: 616-628, <http://dx.doi.org/10.1139/t03-013>
- Feng S., Vardanega P.J., Ibraim E., Widyatmoko I. and Ojum C. (2019). Permeability assessment of some granular mixtures. *Géotechnique*, <https://doi.org/10.1680/jgeot.17.T.039>



Kozeny, J. (1927). Über kapillare Leitung des Wassers im Boden (Aufstieg, Versickerung und Anwendung auf die Bewässerung). *Sitzungsber. Akad. Wiss. Wien.* **136a**, 271–306 (in German)

Vardanega, P. J., Feng, S. and Shepherd, C. J. (2017). Some recent research on the hydraulic conductivity of road materials. In: *Bearing Capacity of Roads, Railways and Airfields (A Loizos, I Al-Qadi & T Scarpas (eds)) Proceedings of the 10th International Conference on the Bearing Capacity of Roads, Railways and Airfields (BCRRA 2017)*, Athens, Greece, June 28-30, 2017, Taylor & Francis, London, UK, pp. 135-142 (full-paper on USB).

### Figure captions

**Figure 13.** Predicted  $k_{20oC}$  versus Measured  $k_{20oC}$  for Eq. (28a), calibrated with the dataset from Feng et al. (2019) ( $n = 30$ ).

Downloaded by [ University of Bristol] on [30/07/19]. Copyright © ICE Publishing, all rights reserved.